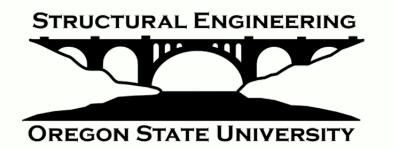


Renewal of Aging and Deteriorated Reinforced Concrete Bridges with Titanium Alloy Bars (TiABs)

Christopher Higgins, Ph.D., P.E. Deanna Kuhlman Laura Baughman Mackenzie Lostra Jonathan Knutdsen Sharoo Shresta Eric Vavra





Overview

- Introduction, Background, and Motivation
- Laboratory Test Results from Full-Scale Specimens Shear Strengthening Flexural Strengthening
- Field Implementation on Mosier Bridge over 184
- Design Guide and ASTM Material Specification
- Conclusions



Introduction

During the 1950 and 60's:

- Post-war construction boom
- Reinforced concrete widely used
- Newly standardized deformed reinforcing steel bars produced poor details
- Design codes were not conservative

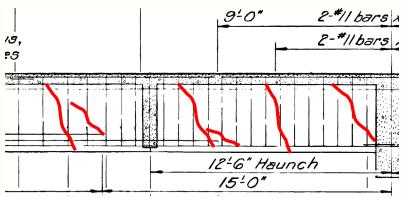
Now:

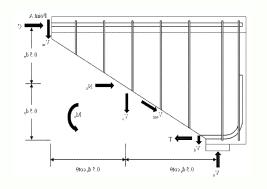
- Visual distress, changes in use, extend life
- Using modern *design* codes to assess

Results:

• Replace, limit loads, retrofit









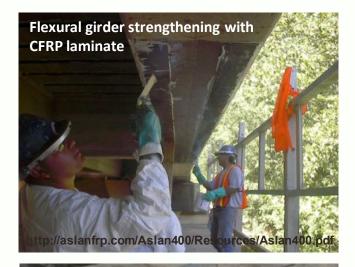
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Strengthening Approaches

- Post-tensioning
- Wrapping/confining
 - Carbon fiber reinforced polymer (CFRP) laminate
- Near-surface mounted (NSM)
 - Carbon fiber reinforced polymer rod/strip
 - Glass fiber reinforced polymer (GFRP) rod
 - Stainless steel bars

FRP rods and laminates fail due to bond and anchorage and materials are nonductile

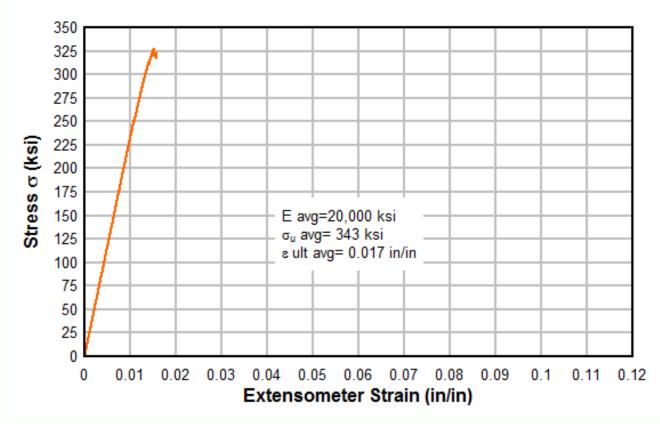
Concerns with corrosion at surface for most metals







Background: NSM Strengthening Materials



Carbon Fiber Reinforced Polymer (CFRP)



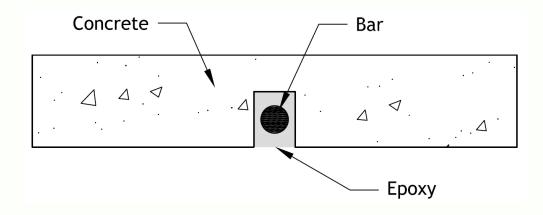
CFRP Bond Failure – Limits material strength





Near-Surface Mounting







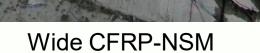




Outer shell peeling

Inner core cracked diagonally





Tightly spaced CFRP-NSM



Alternatives?

Want environmentally insensitive material with high strength, well defined properties, and efficient mechanical anchorages

-> Titanium



Titanium?

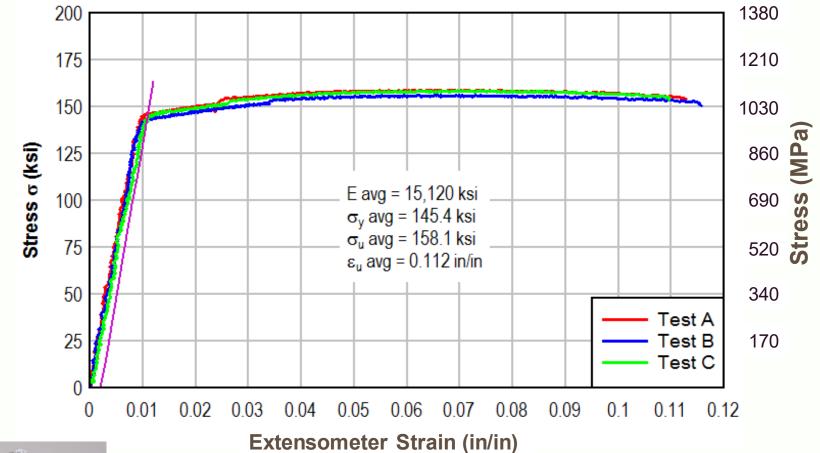
No one uses titanium in structural engineering!

It is too expensive...

It's only for aircraft or medical devices....



Titanium Alloy Material Properties (Ti-6Al-4V)







Titanium Alloy Material Properties (Ti-6Al-4V)

- Aircraft fastener quality (6% Aluminum 4% Vanadium)
- Well-defined, high strength, and ductile (limited hardening->protects bond, structural fuse)
- High fatigue resistance (CAFL~75 ksi), low notch sensitivity
- Impervious to chlorides due to stable oxide layer
- Coeff. of thermal expansion (8.6με/°C) (8-12 Con. and 12 St.)
- Conventional fabrication (shear, cut, and bend)
- **Relatively lightweight** of 281 lb/ft³ (steel 1.7x)
- **Bends** facilitate anchorage



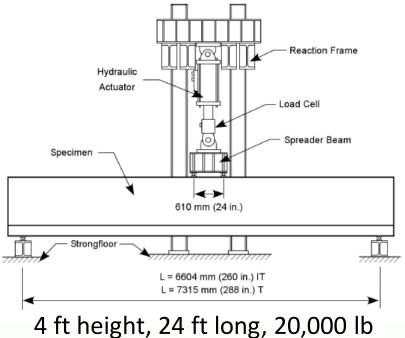
Experimental Work

- Full-scale tests with typical proportions and materials from legacy designs
- Shear specimens: 10

 (3 control)
 1/4 in. diameter TiABs
- Flexure specimens: 10

 (3 control)
 5/8 in. diameter TiABs
- Fatigue and freeze-thaw exposure: 3 (2 shear, 1 flexure)





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Epoxy Properties

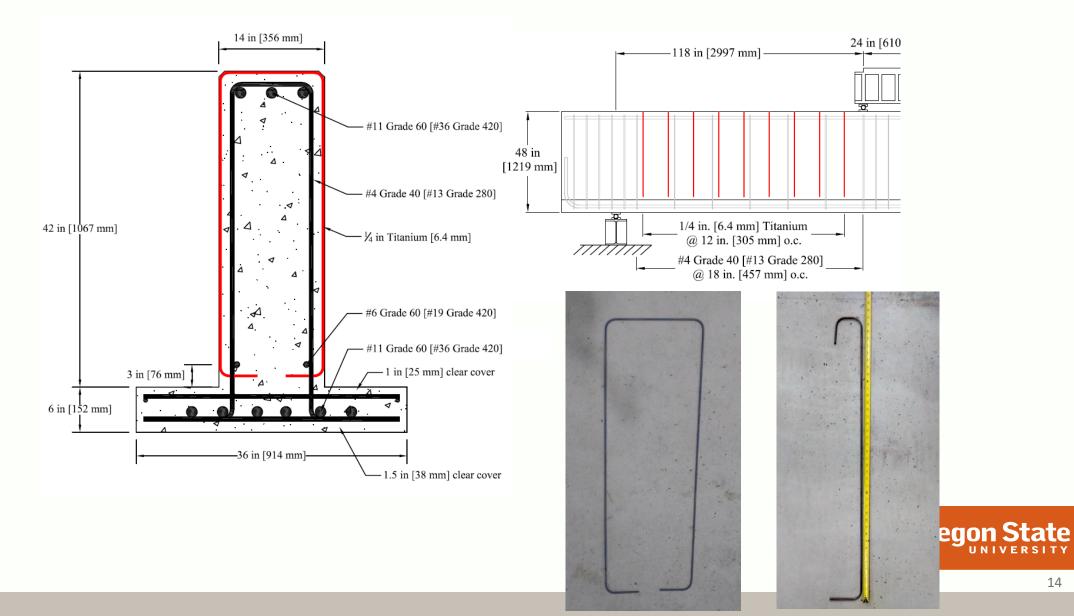
BASF MasterEmaco ADH 1420:Bond = 2000 psiUnitex Pro-Poxy 400:Bond = 2800 psiHilti HIT-RE 500 V3:Bond = 1700 psi







Shear Strengthening – Cross sections (High V and M-)



Shear : Installation







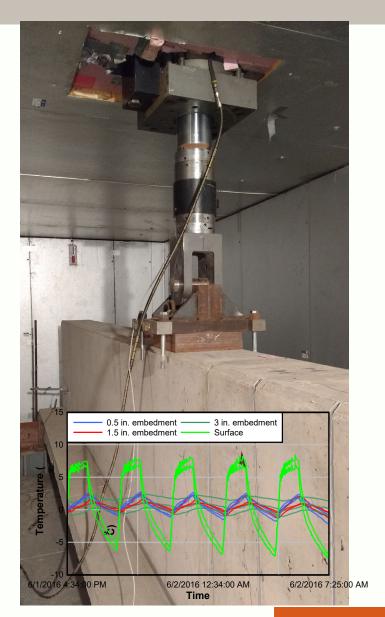
Shear : Fatigue with Freeze-Thaw

- Designed to simulate 50 years of damage based field testing
- 2,400,000 cycles
- Internal stirrup stress range of 13 ksi

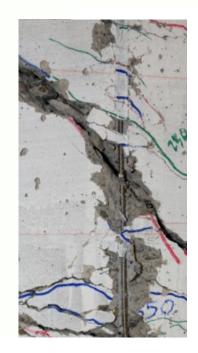
$$SR_{eqv} = \sqrt[3]{\sum \frac{n_i}{N_{tot}} SR_i^3}$$

Freeze-Thaw

- 120 cycles
- Represents 25-100 years of damage in Oregon, depending on location



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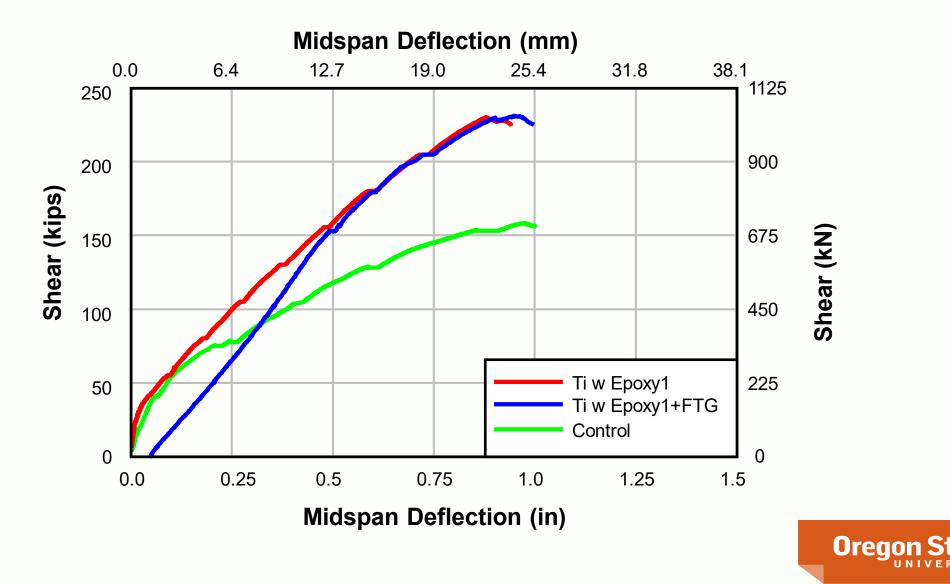






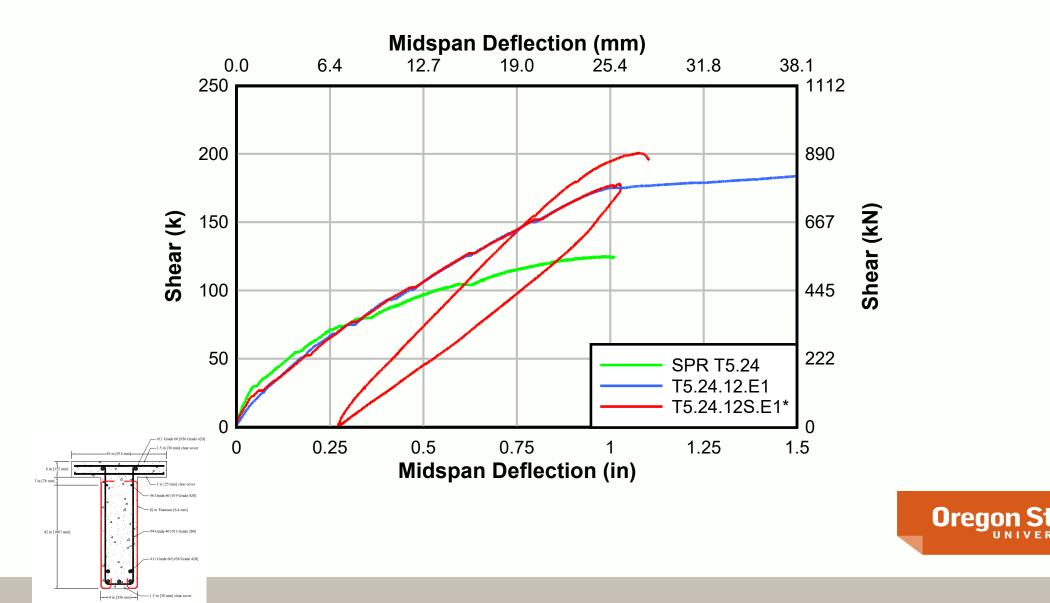


Shear Results Epoxy E1 Ti@ 12 in.



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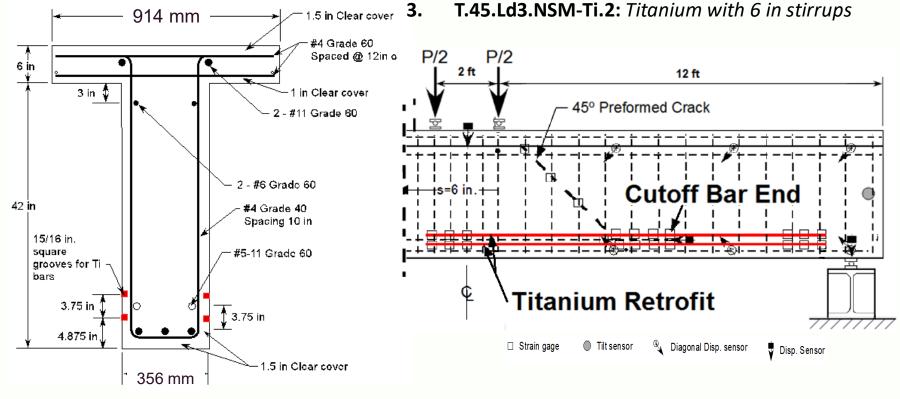
T Specimens Load-Deflection

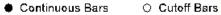


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Flexure T Beam Details

- 1. T.45.Ld3: Baseline T Beam
- 2. T.45.Ld3.NSM-Ti: with 10 in stirrups

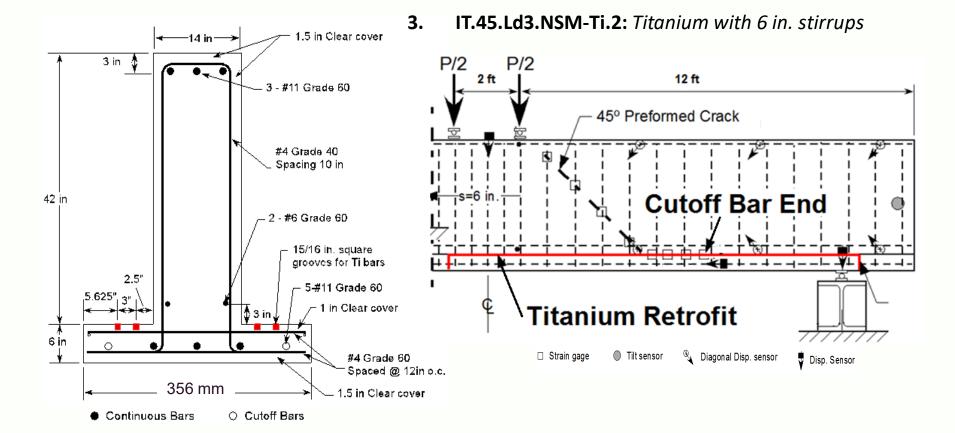




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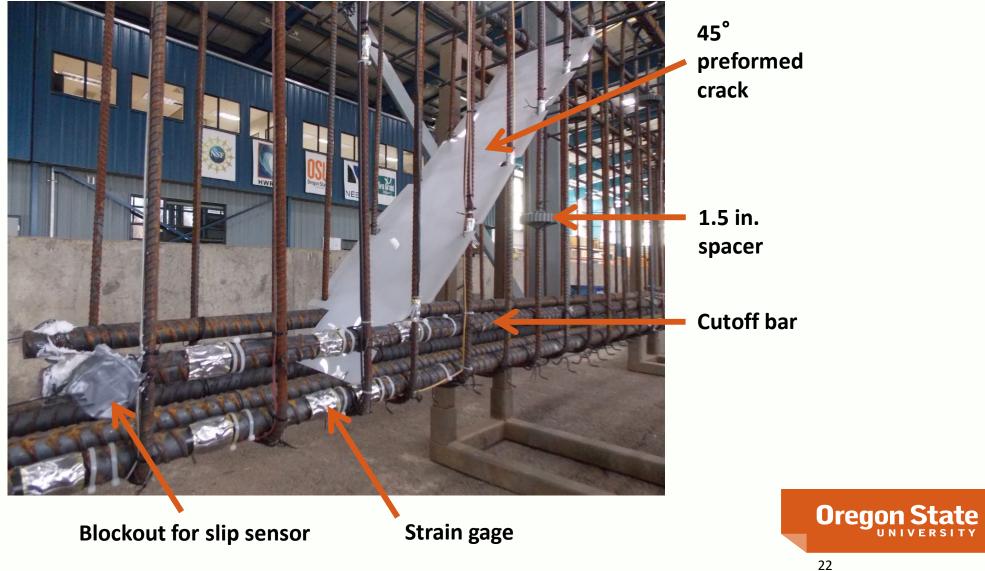
IT Beam Details

- 1. IT.45.Ld2: Baseline IT Beam
- **2. IT.45.Ld3.NSM-Ti:** *Titanium with 10 in. stirrups*



Oregon State

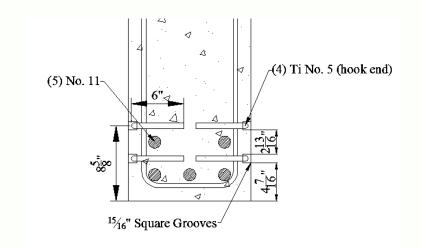
T and IT Beam Construction



Experimental Setup: NSM Strengthening Methodology

ACI 440.2R

- Groove Spacing
- Groove dimensions



Epoxy Manufacturer Data





Tensile	Elongation at	Compressive Yield	Bond Strength
Strength (ksi)	Break (%)	Strength (ksi)	(2 day cure) (ksi)
4	1	12.5	



Experimental Setup: NSM Strengthening Methodology

Hook Fabrication

- 2 Ti bars on each side
 - 12.5 ft length
 - 6 in. hooks
- 2 in. bend diameter
- Ti: Heat to 900 $^\circ F$ or 1250 $^\circ F$

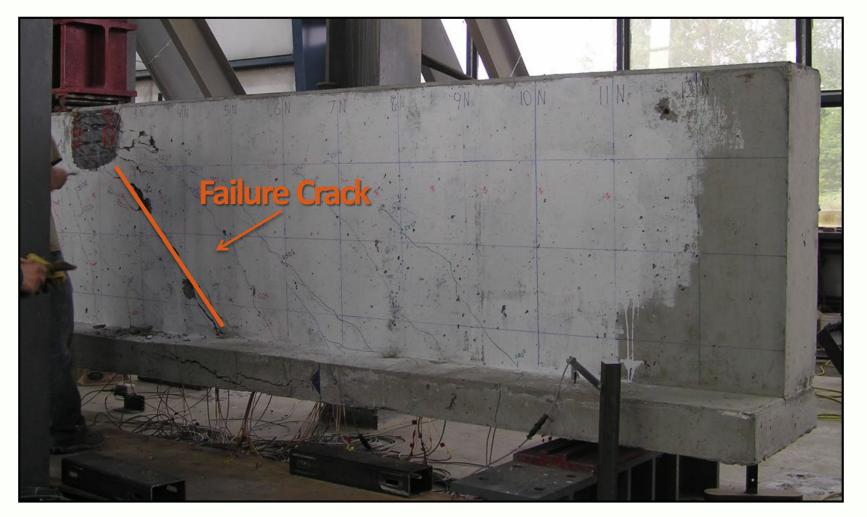






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IT.45.Ld2 Failure



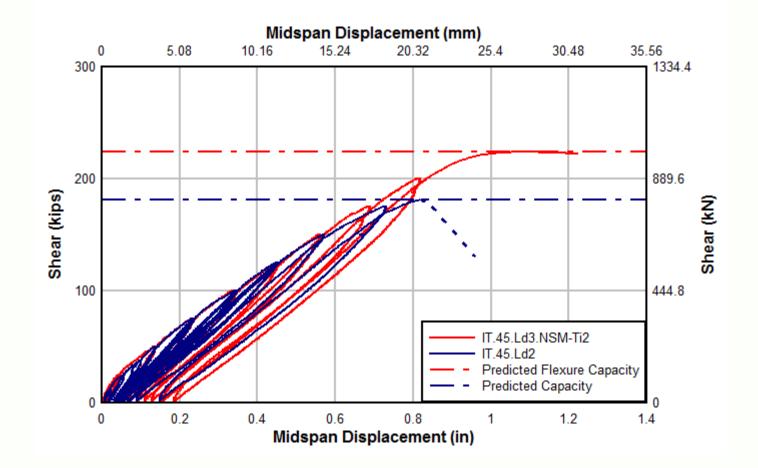


IT.45.Ld3.NSM-Ti2 Failure

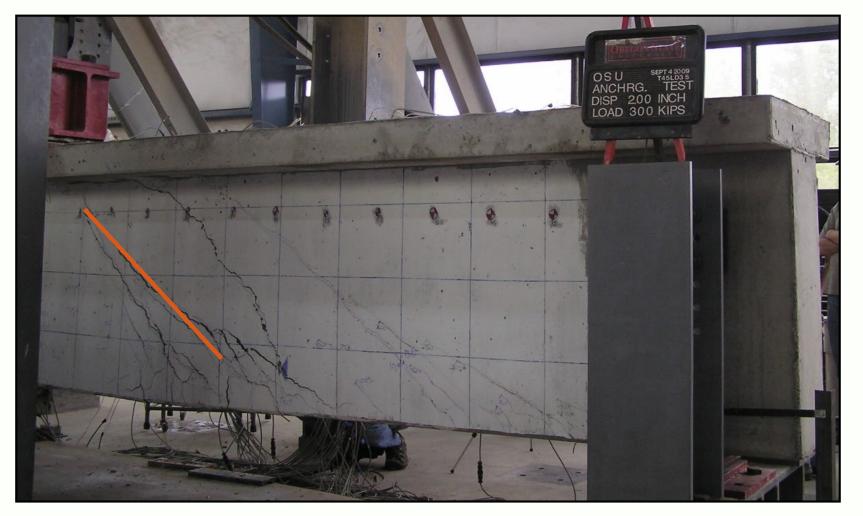




IT Beam Experimental Results

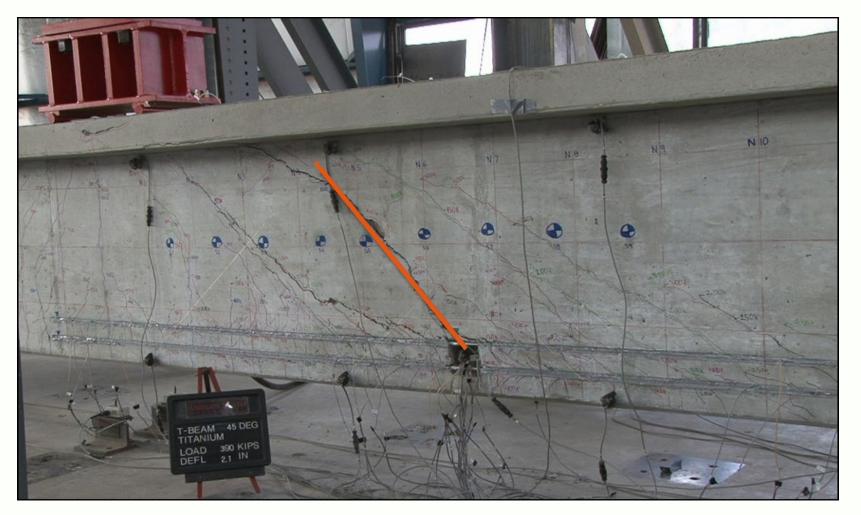


T.45.Ld3 Failure





T.45.Ld3.NSM-Ti Failure



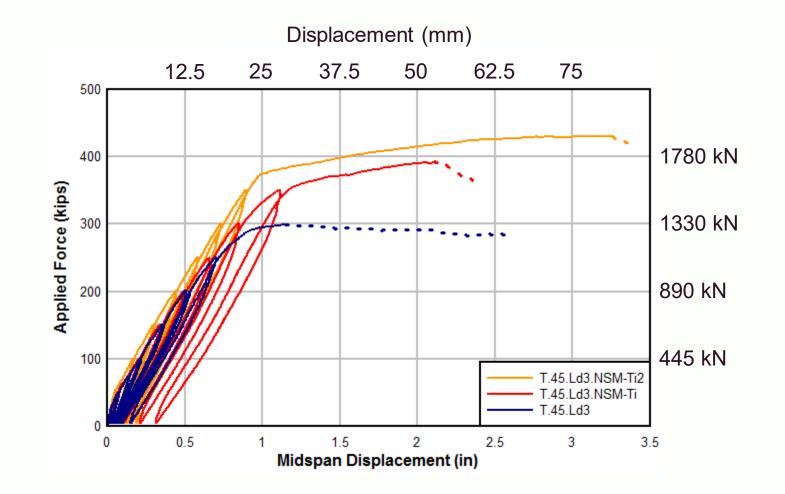


T.45.Ld3.NSM-Ti2 Failure





T Beam Experimental Results



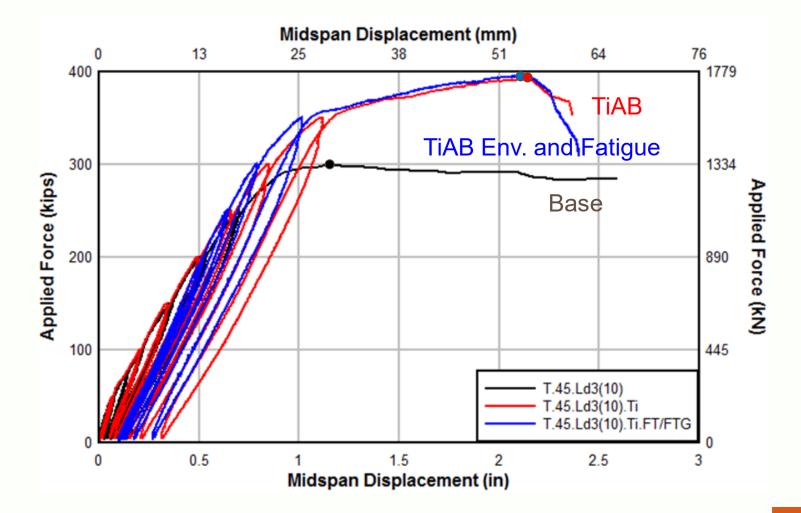
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Durability High Cycle Fatigue and Freeze-Thaw Combined

- Largest combined structural-environmental chamber
- Thermocouples at 0.5, 1.5, and 3 in. ensure temperature targets
- 1.6 million cycles @ steel stress range >50 years of life.



T Beam Experimental Results – Durability (s=10 in.)



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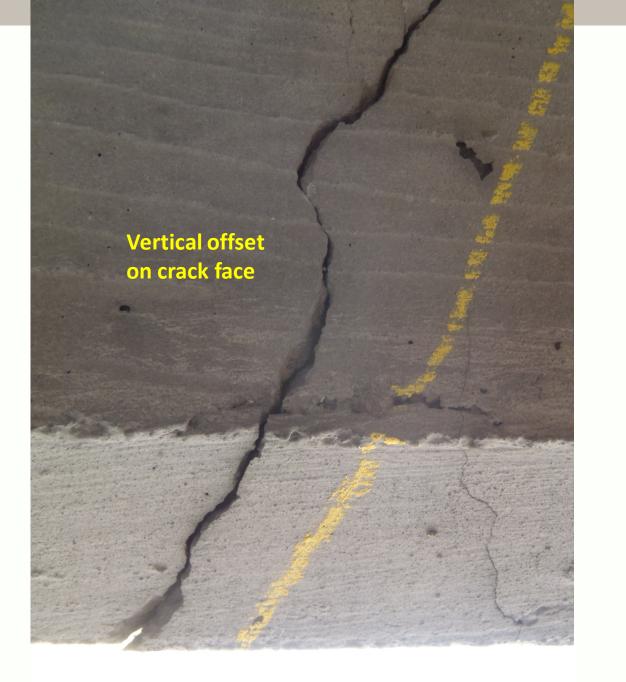
Mosier Overcrossing of Interstate 84

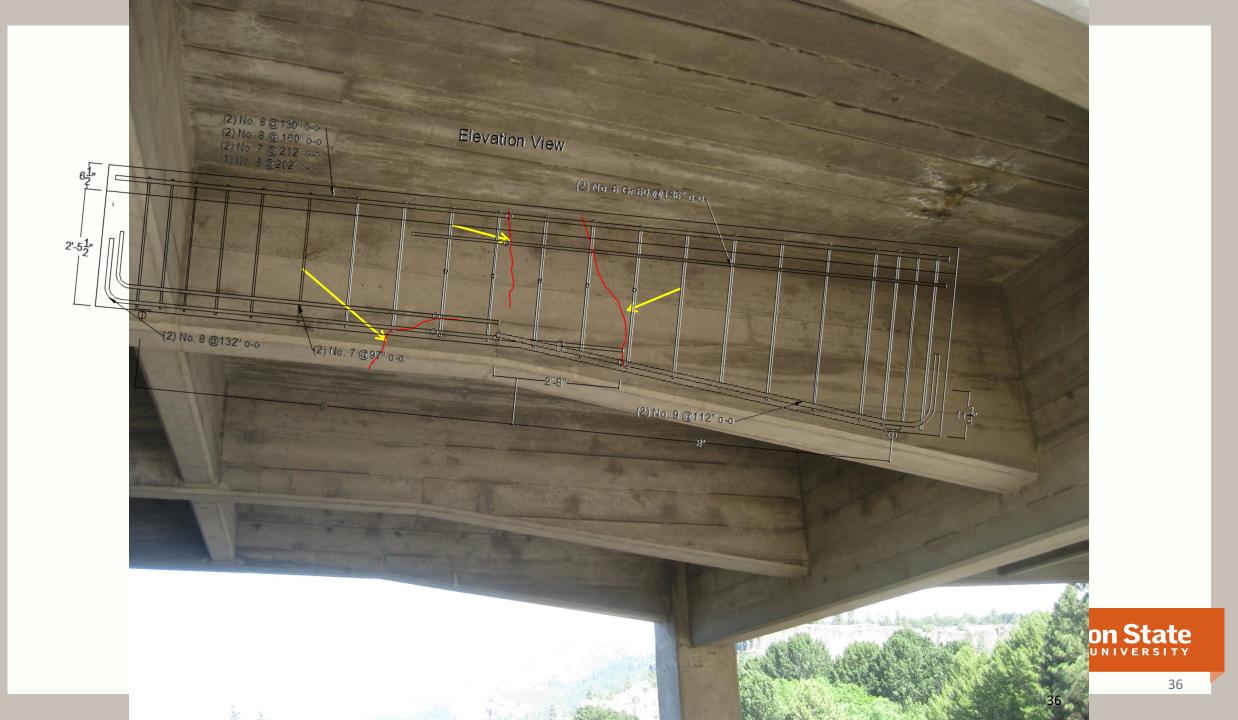
- Built in 1952
- Serves a nearby quarry



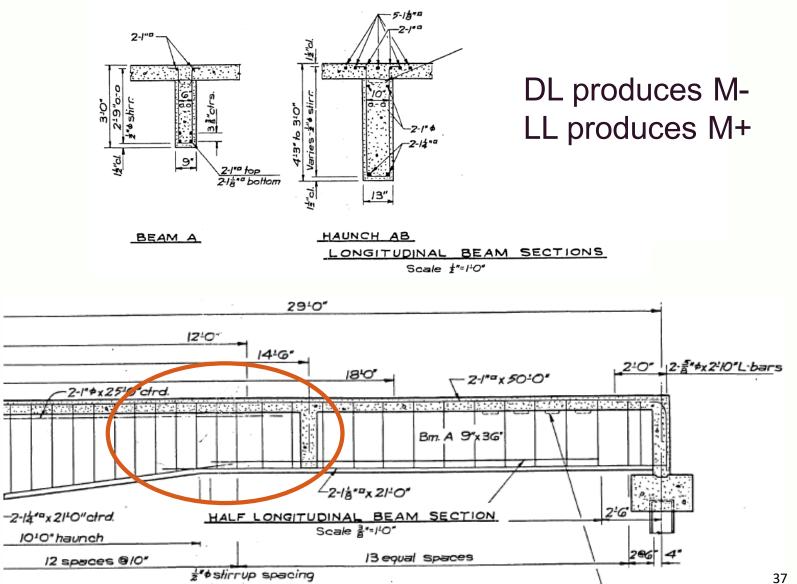








Mosier As-Built Details



Test Plan

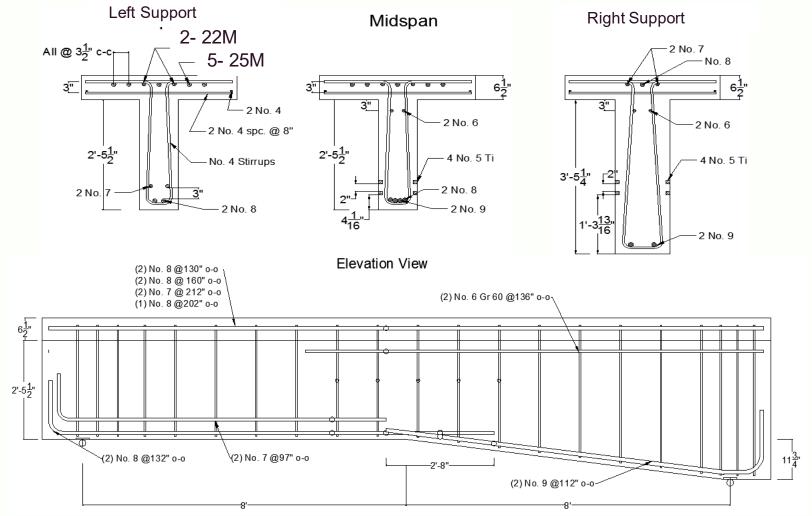
Three specimens:

- **1.** Mosier 1: As-Built
- **2. Mosier 2**: Strengthen after failing reinforcing steel anchorage (designer's assumption)
- **3.** Mosier **3**: Strengthen with reinforcing steel anchorage intact

Searched mill certifications to locate bars that best matched strength curves of original design. Used smaller sized Grade 420 (60) rebar to match development length of intermediate grade steel (280 MPa (40 ksi))

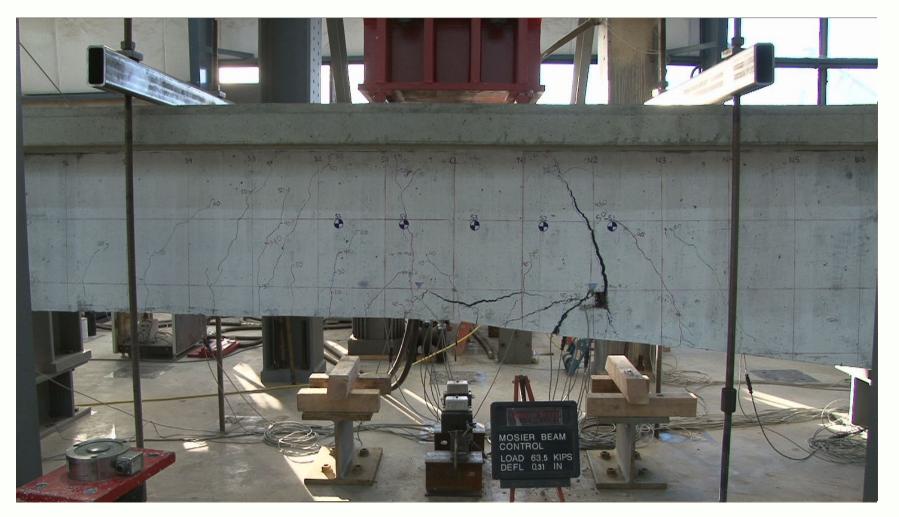


Mosier Beam Details



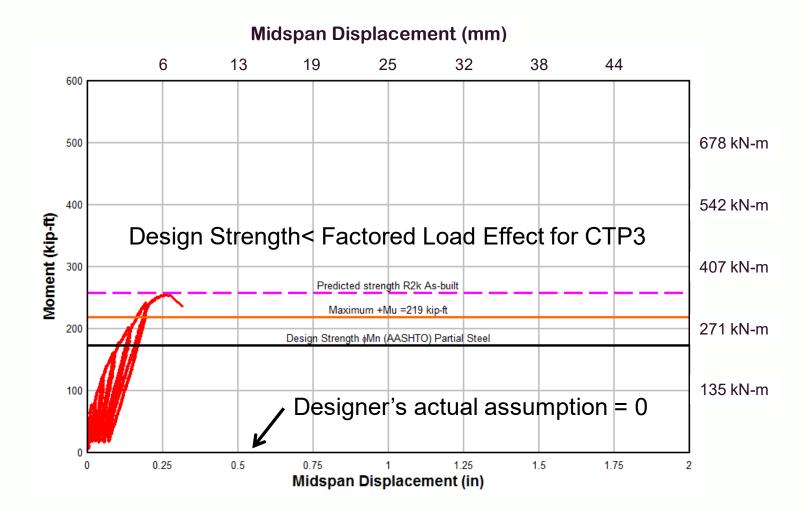
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Experimental Results: Mosier 1





Experimental Results: Mosier 1



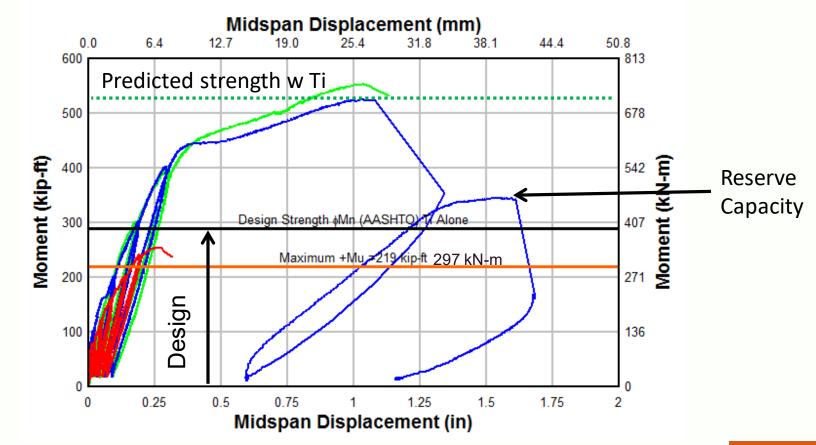
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Experimental Results: Mosier 3



Analysis

- Reserve strength of Ti girder substantially exceeds factored demands
- Failed anchorage provided similar response as intact



• Design strength of Ti girder exceeds factored demands even with conservative assumptions

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30% less expensive than CFRP



ASTM Specification for NSM Titanium

Main Committee: Committee B10 – Reactive and Refractory Metals and Alloys

Sub-Committee: Committee B10.01 on Titanium



Approved Nov. 2018



Designation: B1009 – 18

Standard Specification for Titanium Alloy Bars for Near Surface Mounts in Civil Structures¹

This standard is issued under the fixed designation B1009; the number immediately following the designation indicates the year of original adoption or, in the case of revision, the year of last revision. A number in parentheses indicates the year of last reapproval. A superscript epsilon (ε) indicates an editorial change since the last revision or reapproval.

1. Scope

1.1 This specification covers titanium alloy bars with surface deformations and 90-degree anchorage hooks for use as near surface mounts for flexural and shear strengthening of concrete beams. The product can be furnished with or without anchorage hooks as specified by the purchaser. If supplied without hooks, the hooks shall be bent on-site prior to installation, as this method requires two 90-degree anchorage hooks.

1.2. The values stated in inch-nound units are to be regarded

- D7913/D7913M Test Method for Bond Strength of Fiber-Reinforced Polymer Matrix Composite Bars to Concrete by Pullout Testing
- E8/E8M Test Methods for Tension Testing of Metallic Materials
- E29 Practice for Using Significant Digits in Test Data to Determine Conformance with Specifications
- E539 Test Method for Analysis of Titanium Alloys by X-Ray Fluorescence Spectrometry
- E1409 Test Method for Determination of Oxygen and Nitro-

ASTM B1009-18 Requirements:

- Tensile properties
- Chemical requirements
- Bond strength
- Cross-Sectional area calculation
- Bending requirements



Design Guide Available

- "Guide for Design and Construction of Near-Surface Mounted Titanium Alloy Bars for Strengthening Concrete Structures"
- AASHTO-LRFD Format
- General Conditions
- Materials
- Construction
- Installation
- Design
 - Flexure and Shear (MCFT)



Design Guide

- Conventional analysis methods
- Design TiABs at yield if conditions are met
- Includes environmental durability factor (epoxy)
- 3 Limit states for flexure and 1 for shear
 - Strength
 - Service (check bond stress at cutoffs and where retrofitted strength above base capacity)
 - Fatigue (not of TiAB but of reinforcing steel)
- Comprehensive design example (shear and flexure)

Conclusions

Titanium Alloy Bars (TiABs) provide

- Well-defined material properties
- High strength
- Ductility
- Environmental durability and
- Ability to fabricate mechanical anchorages

These attributes make the Ti-6Al-4V alloy reinforcement a promising material for *economically* strengthening bridges and other structures.



Acknowledgements

- Oregon Department of Transportation
 - Bruce Johnson, TACs, and Research group
- Perryman Company, Houston, PA

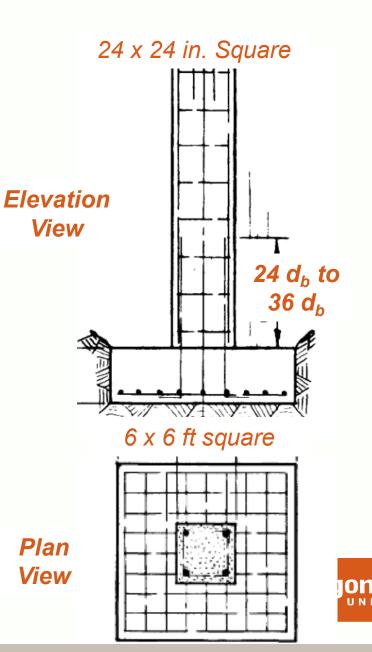
 Undergraduate Research Assistants: Kyle Logan, Jonathon Roy, Aléxia Ribeiro, Lance Parson, Hunter Anderson, Kyle Sonnevile, John Huntoon, Glen Galant, Corey Groshong, James Kemp, and Spencer Maunu

The findings and conclusions are those of the author and do not necessarily reflect those of the project sponsors or the individuals or companies acknowledged.

Ureq

Seismic Deficiencies of pre-1970's columns

- Insufficient transverse reinforcement
 - #3 a@ 12 in spacing
- Common design details:
 - Lap-splice lengths of 24 d_b to 36 d_b
 - Large bar sizes (> #11; square and round)
 - Longitudinal rebar placed at column corners
 - Grade 40 steel (275 MPa)
 - f'_c = 3300 psi (22.7 MPa)



Common Approach for Retrofitting

Fiber reinforced (FRP) laminates (Confinement)

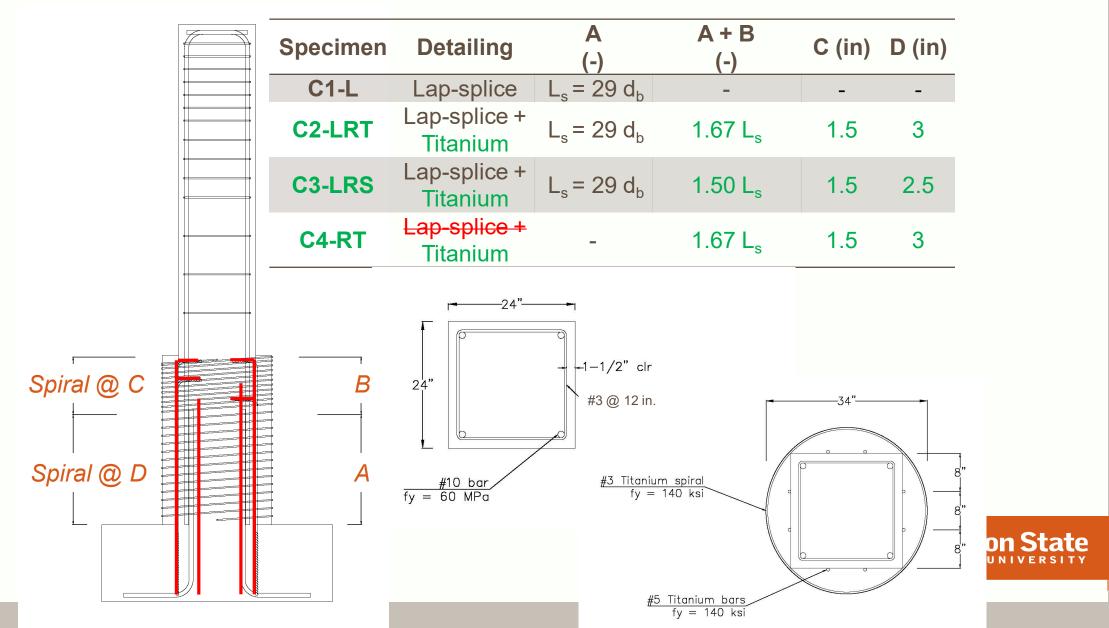
- High-strength
- Surface preparation
- Non-ductile
- Degradation concerns
- Not inspectable



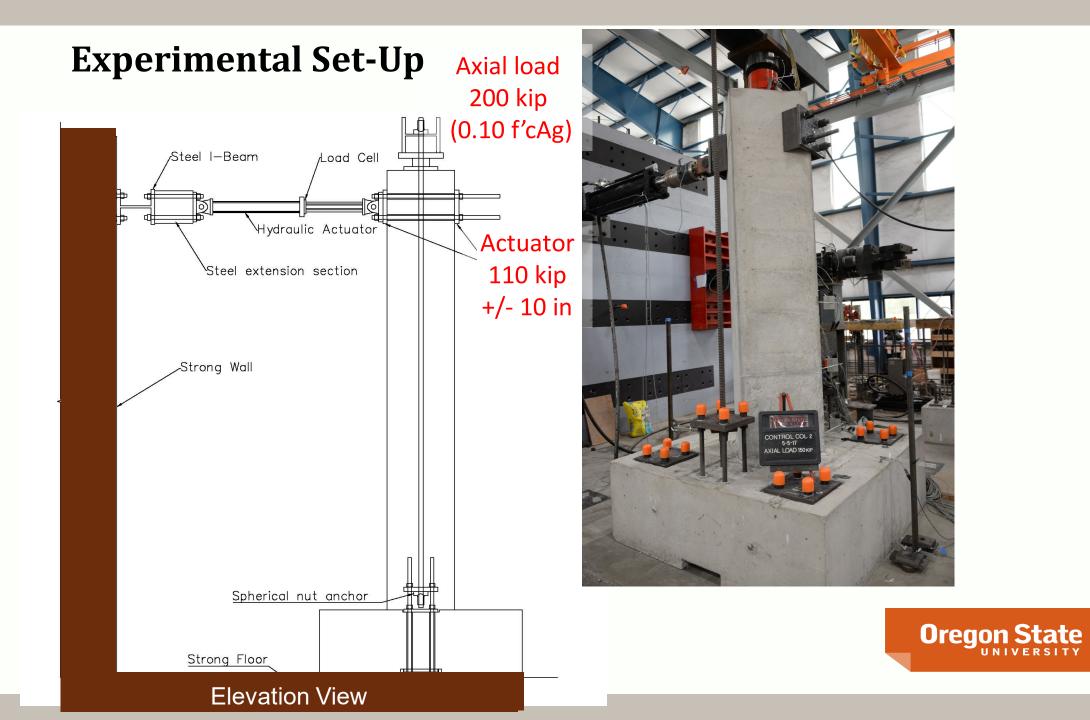




Seismic Performance



54



TiAB Spiral Reinforced Concrete Shell

- Continuous spiral
- Debonded shell from column with plastic sheet
- Flexible polycarbonate sheet formwork
- Ratchet strap drawn tight to TiAB spiral (no cover) and holds form
- See-through, so know completely filled



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Control Specimen: Observed Performance

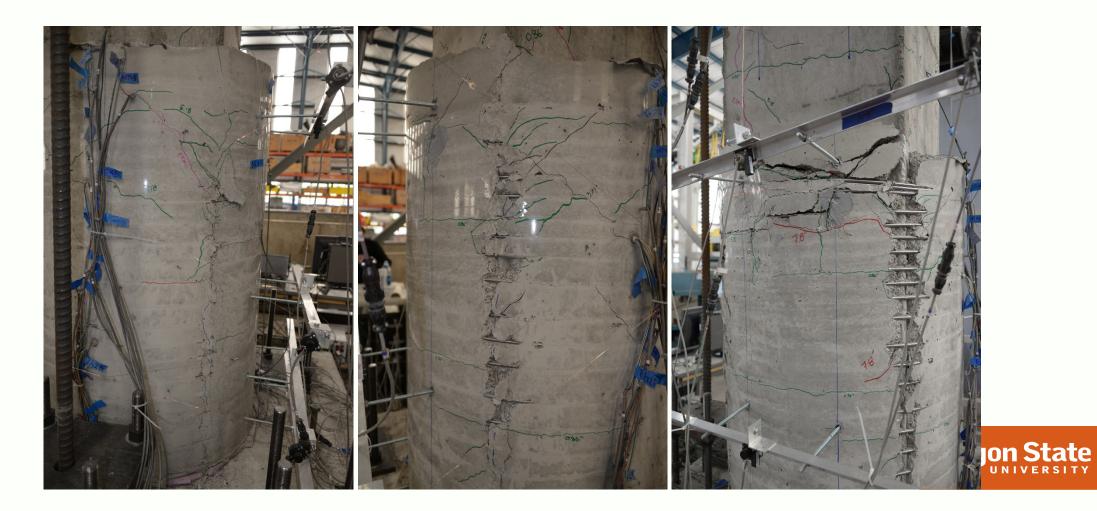
Progression of lap-splice exposure and bond-slip

- Lap-splice failure -> rapid flexural strength degradation
- Severe spalling
- Non-ductile



Titinium Observed Performance

Retrofitted specimens: corner spalling progression



Observed Performance

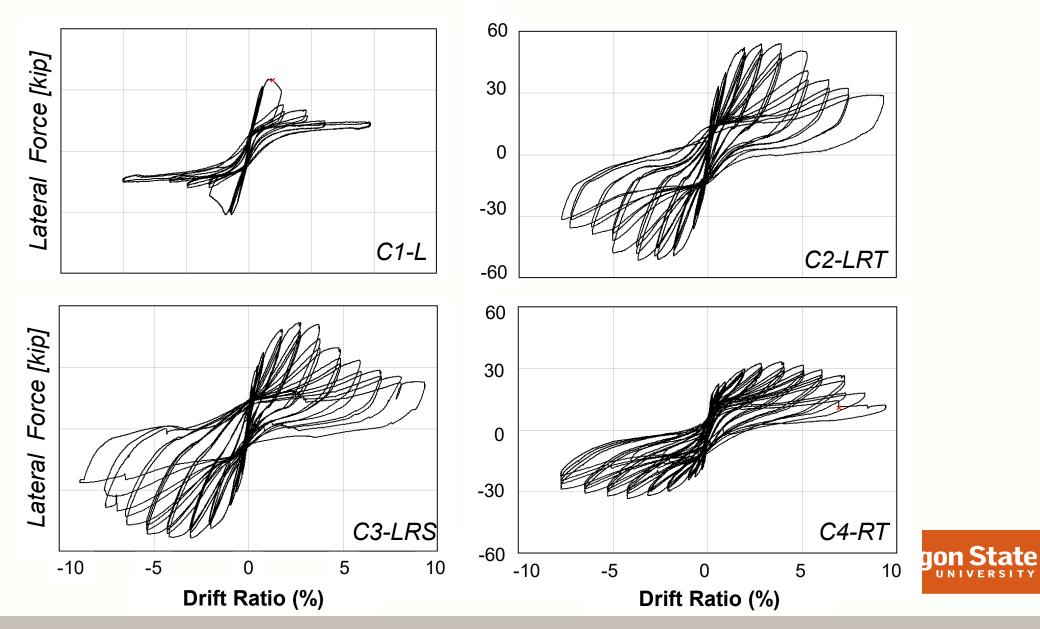
Retrofitted specimens with lap splices (similar performance):

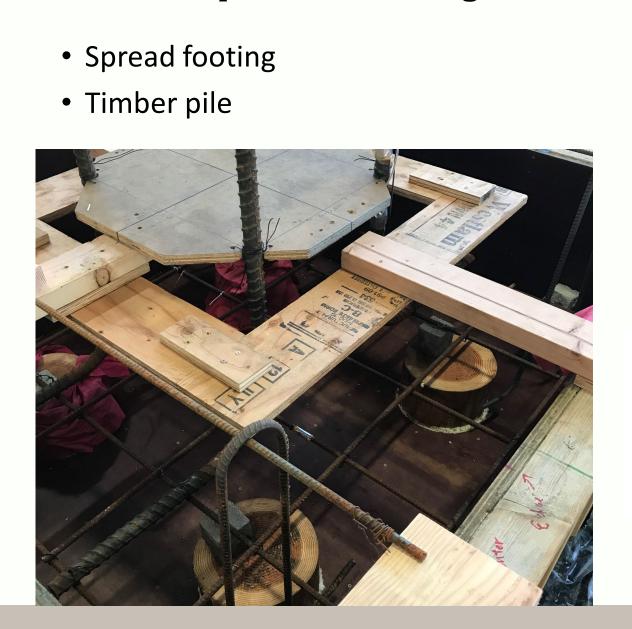
- Ductile withdrawal of hooked anchorages
- Footing concrete spall cones
- Rocking column behavior

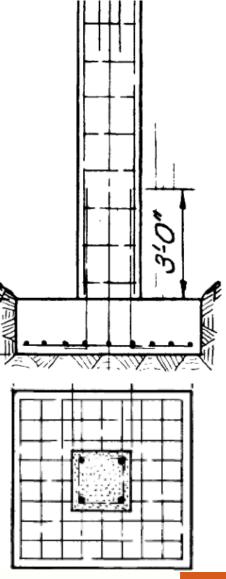




Load-Deformation Response







Fuse Seismic Forces Imparted on Footing



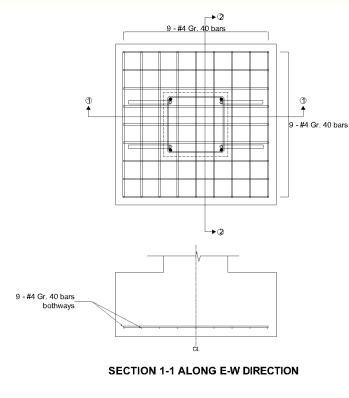
Experimental Tied Footing Details

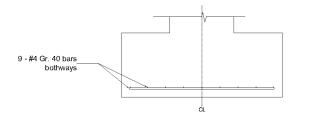


Force column failure But do not reflect in situ details or reactions

Typical Footing Details

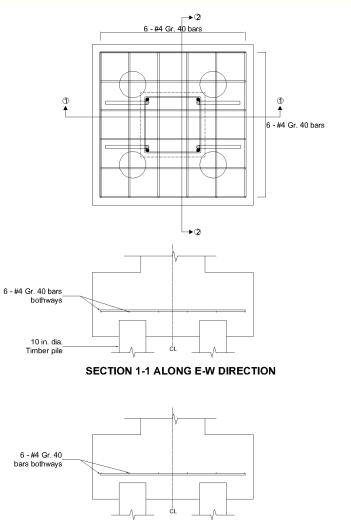
Typical Spread Footing





SECTION 2-2 ALONG N-S DIRECTION

Typical Timber Pile Footing



SECTION 2-2 ALONG N-S DIRECTION

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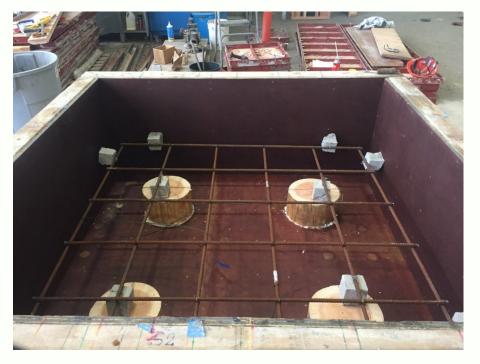
UNIVERSITY

Typical Footing Details

Typical Spread Footing



Typical Timber Pile Footing



Final 2 specimens



Conclusions

Titanium's

- Well-defined material properties
- High strength
- Ductility
- Environmental durability and
- Ability to fabricate mechanical anchorages make the Ti-6Al-4V alloy reinforcement a promising material for economically strengthening bridges for gravity loads and achieving high seismic performance of poorly detailed bridge columns.

Orea

Acknowledgements

Oregon Department of Transportation

• Perryman Company, Houston, PA

 Undergraduate Research Assistants: Kyle England, Brandon Zaikoski, Caleb Lennon, Liam Kucey, Tyler Redman, Anthony Quinn, and Jonathan Roy

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